

### Loading

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It is important to note that, per NEMA Standard VE1, cable tray is not designed to support personnel. The user should display appropriate warnings to prevent the use of cable tray as walkways.

#### Cable Loads

The cable load is the total weight, expressed in kg/m or lb/ft, of all the cables that will be placed in the cable tray.

#### Seismic Loads

It is now known that cable tray systems can withstand stronger earthquakes than previously thought. The tray itself and the support material are highly ductile, and the cables moving within the tray tend to dissipate energy. However, if you have specific seismic specifications for selected cable tray, please consult T&B to ensure your specifications are met.

## Load Diagrams for Beams

### CANTILEVER BEAMS

#### Uniform Load

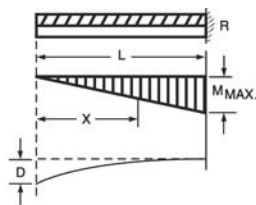
w per unit of length: total load  $w$   
Reaction  $R = wL = W$

$$\text{Moment at any point: } M = \frac{wX^2}{2} = \frac{WX^2}{2L}$$

$$\text{Maximum moment } M_{\text{max}} = \frac{wL^2}{2} = \frac{WL}{2}$$

$$\text{Maximum deflection, } D = \frac{wL^4}{8EI} = \frac{WL^3}{8EI}$$

$$\text{Maximum Shear, } V = wL$$



#### Concentrated Load at Free End

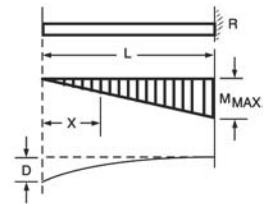
Reaction;  $R = P$

Moment at any point:  $M = Px$

Maximum moment,  $M_{\text{max}} = PL$

Maximum deflection,  $D = \frac{PL^3}{3EI}$

Maximum Shear,  $V = P$



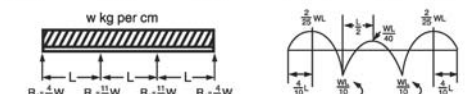
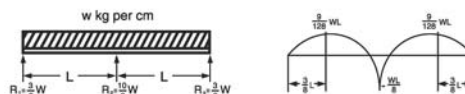
### CONTINUOUS BEAMS

#### Two Span

$W = wL$

$R = \text{Reaction, kg}$

$L = \text{Span Length, cm } R_1 = cw$

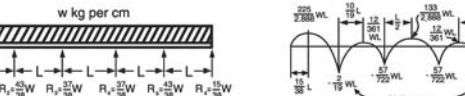


#### Three Span



#### Four Span

#### Five Span



### SIMPLE BEAMS

#### Uniform Load

w per unit of length, total load  $w$

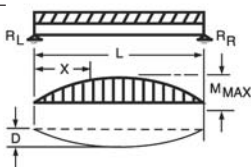
ReactionS:  $R_L = R_R = \frac{wL}{2} = \frac{W}{2}$

Moment at any point:  $M = \frac{wX(L-X)}{2} = \frac{WX(L-X)}{2L}$

Maximum moment, AT CENTRE  $M_{\text{max}} = \frac{wL^2}{8} = \frac{WL}{8}$

Maximum deflection:  $D = \frac{5wL^4}{384EI} = \frac{5WL^3}{384EI}$

Maximum Shear:  $V = \frac{wL}{2}$



#### Concentrated Load at Center

Reaction  $R_L = R_R = \frac{P}{2}$

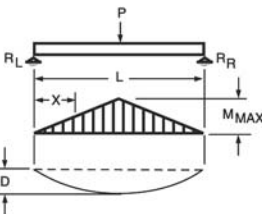
Moment at any point:  $X \leq \frac{L}{2}, M = \frac{PX}{2}$

$X > \frac{L}{2}, M = \frac{P(L-X)}{2}$

Maximum moment, At Center,  $M_{\text{max}} = \frac{PL}{4}$

Maximum deflection,  $D = \frac{PL^3}{384EI}$

Maximum Shear,  $V = \frac{P}{2}$



#### Concentrated Load at any Point

Reaction:  $R_L = \frac{Pb}{L}, R_R = \frac{Pa}{L}$

Moment at any point:  $X \leq a, M = R_L X = \frac{PbX}{L}$

$X > a, M = R_R (L-X) = \frac{Pa(L-X)}{L}$

Maximum moment, At  $X = a, M_{\text{max}} = \frac{Pab}{L}$

Maximum deflection,  $D = \frac{Pab(L+b)3a(L+b)}{27EI L}$

Maximum Shear,  $V = \frac{Pa}{L}$ , WHEN  $a > b$

